#### **Contents**

- Step 1: Initialization of workspace
- Step 2: Defining Property
- Step 3: Generation of Floor plan
- Step 4: Generation of Wind Braces
- Step 5: Generation of storey
- Step 6: Generation of Truss
- Step 7: Boundary Conditions
- Step 8: Creating Load Cases
- Step 9: Perform Analysis
- Step10: Results
- Step11: Design Input & Output

# **Steel Structure**

This Tutorial presents an efficient method of modeling and analyzing a plant structure.

The Install CD provides an animation illustrating all the modeling, analysis and results verification processes of the present example with concurrent vocal explanations. The tutorials are much easier to follow if the entire process of analysis is first understood through the narrated animations.

The step-by-step modeling and analysis processes presented in this example are the following:

Program Version	Gen 2024 (v2.1)
Revision Date	Mar. 19, 2025



Step

00





Applied Code	Materials
Eurocode	S 460

Platf	orm	Sor	tion
i iau		UCU	

Section ID	Section type	Description
1	HEA 400	Exterior Columns, Roof level girder
2	HEA 500	Interior Columns
3	HEA 260	Floor girders/ beam

#### **Applied Load**

Load	Description	Intensity
Dead Load	Self Weight	
Live Load	Outer Girder Inner Girder	11.5 kN/m 5.6 kN/m
Roof Load	Wind Load	5.4 kN/m
Wind Load	Wind X (inner) Wind X ( outer) Wind Y (inner) Wind Y(outer)	4.9 kN/m 9.8 kN/m 2.7 kN/m 5.5 kN/m

### 1-1. Initialization of workspace



# 1-2. Generation of Line Grid



5

# 2-1. Defining the Material Type.

Duccostan		1 Material Data				×
Procedure	r -	General				
Manue Madale Dranautiane Matarial	Properties	× Material ID	1	Nam	e \$460	
wenu>wodei>Properties>wateriai	Material Section Thickness				-	
		Elasticity Data				
	ID Name Type Standard DB Add	Type of Design	Steel	<ul> <li>Steel</li> </ul>	TH(C)	
Material ID :1	1 S460 Steel EN(S) S460 Modify			Star	EN(S)	¥
Type of Design · Steel	Delete				JB 5400	
Standard Code EN(S)				Pro	duct	
Type of steel + \$460	Сору			Conc	rete	
Type of Steel . 5460	Import	_		Star	ndard	~
	Renumbe	r Type of Material	Orthotronia		Code	~
		U ISOU OPIC	Orthouropic	r i	DB	~
		Steel				
		Modulus of Elasti	city: 2.10	000e+08 kN/m²		
		Poisson's Ratio	÷	0.3		
		Thermal Coeffici	ent : 6.66	667e-06 1/[F]		
		Weight Density		76.98 kN/m3		
				7.85 kN/m3/c		
	Clos		sity:	KN/11-/9		
		Concrete		0000+00		
		Modulus of Elasti	city: 0.00	kN/m <sup>2</sup>		
		Poisson's Ratio	:	0		
		Thermal Coeffici	ent : 0.00	00e+00 1/[F]		
		Weight Density	:	0 kN/m <sup>3</sup>		
		Use Mass Der	isity:	0 kN/m³/g		
		Plasticity Data				
		Plastic Materia	NONE NONE		×	
		Inelastic Material F	roperties for Fiber M	lodel & Non-dissipa	tive element	
		Concrete N	one	<ul> <li>Steel</li> </ul>	None	~
		Confined Concrete	e for Columns	None		~
		Thermal Terrefor				
		Thermai Transfer	0			
		Specific Heat	: 0	Btu/kN·[	FJ	
		Heat Conduction	: 0	Btu/m·h	r•[F]	
		Damping Ratio	: 0.02			
				ОК	Cancel	Apply

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# 2-2. Defining Frame Section Properties.

Procedure	Section Data				×	Properties	×
Defining Frame Section Properties	DB/User Value SRC Comb	ined Tapered Co	omposite			2 Section Thickness	1
Menu>Model>properties>Material properties>Section	Section ID 1Name HEA400	I-Section	DB U	INI	~	ID     Name     Type     Shape       1     Exterior column     DB     I       2     Interior column     DB     I       3     Floor girders     DB     I       4     Top and bettern column     User     21	Add Modify Delete
Section ID:1			LIE & 400			5 Roof Bracing User L 6 Diagnoal members User 21	Сору
I Section		Sect. Name	Built-U	Jp Section		7 Wind braces User 2L	Renumber
Name: Exterior Columns,	H r1 r2			op beenen			1,2
Roof level girder	1 4/2	Get Data from	n Single Angle				
Section Name: HEA400	►======	DB Name	AISC10(	US)	~		₽—> y
Click [Apply] > [Close]		Sect Marrie					
	2	Н В1	0.39 0.3	m m		H:0.39 B1:0.3 tw:0.011 tf1:0.019 B2:0 tf2:0 r1:0.0	Close
2 Similarly define the other		tw	0.011	m			
Frame Section Properties.	<b>□</b> ⊸ y	tf1 82	0.019	m			
		tf2	0	m			
	4 3	r1	0.027	m			
		r2	0	m			
	Offset : Center-Center Change Offset	Cons	ider Shear Defo ider Warping Ef	ormation. ffect(7th DOF)			
	Show Calculation Results.		ок	Cancel	Apply		

# 2-2. Defining Frame Section Properties.

Procedure	Section Data		X Section Da	ta		×
Defining Frame Section Properties	DB/User Value SRC Comb	ined Tapered Composite	DB/User	Value SRC Combined Tape	ared Composite	
Menu>Model>properties>Material properties>Section	Section ID 2 Name Interior column	User O DB UNI	Section Name	D 3 I-S Floor girders Ouser	ection DB UNI	~
Section ID:2 I Section		Sect. Name HEA500 Built-Up Section		-81	Name HEA260	~
Name: Interior Columns, Section Name: HEA500 Click [Apply]		Get Data from Single Angle DB Name Sect. Name		Get Da B2-4 Get Da B8 Na Sect.	ata from Single Angle .me <b>AISC10(US)</b> Name	~
<ul> <li>SectionID:3</li> <li>I Section</li> <li>Name: Floor girders,</li> <li>Section Name: HEA260</li> <li>Click [Apply]</li> </ul>	y y	H0.49mB10.3mtw0.012mtf10.023mB20mtf20mr10.027mr20m		2 	H     0.25     m       B1     0.26     m       tw     0.0075     m       tf1     0.0125     m       B2     0     m       tf2     0     m       r1     0.024     m       r2     0     m	
	Offset : Center-Center Change Offset	Consider Shear Deformation.	Offset :	Center-Center hange Offset	Consider Shear Deformation.	)
	Show Calculation Results.	OK Cancel	Apply Sho	w Calculation Results	OK Cancel	Apply

Procedure	Section Data		× Section Data		×
Defining Frame Section Properties	98/User Value SRC Combin	ned   Tapered   Composite	3 9/User Value SRC Con	nbined   Tapered   Composite	
Menu>Model>properties>Material properties>Section	Section ID 4	Double Angle     Viser     DB     UNI	Section ID 5 Name Roof Bracing	Angle User DB UNI	>
1 Section ID:4		Sect. Name	10	Sect. Name	
Double Angle Section Name: top and bottom chord, Section : User defined Provide the section values as per the image Click [Apply]		Get Data from Single Angle DB Name UNI Sect. Name H 0.11 m B 0.11 m tw 0.012 m tf 0.012 m C 0 m		Built-Up Section         Get Data from Single Angle         DB Name       UNI         Sect. Name         H       0.1         m       m         tw       0.01         tf       0.01         Consider Principal Axis         the text       0	× ×
<ul> <li>Section ID:5         Angle Section         Name: Roof Bracing,         Section : User defined         Provide the section values as per         the image         Or the section value of the</li></ul>	Offset : Center-Center Change Offset	Consider Shear Deformation.	Offset : Center-Center Change Offset	Consider Shear Deformation.	
Click [Apply]	Show Calculation Results	OK Cancel Apply	Show Calculation Result	S OK Cancel	Apply

#### **Procedure**

Defini	ing Frame Section Properties	Section Data X		Section Data			
Menu: prope	>Model>properties>Material rties>Section	B/User Value SRC Combi Section ID 6	ned Tapered Composite	~	B/User Value SRC Combined     Section ID 7	Tapered Comp	osite
Sect Doul Nam Sect Prov the Click	tion ID:6 ble Angle Section ne: Diagonal members, tion : User defined vide the section values as per image < [Apply]		User     DB     UNI       Sect. Name     Image: Constraint of the section       Get Data from Single Angle       DB Name     UNI       Sect. Name       H     0.1       B     0.1       tv     0.008       tf     0.008       C     0       m			Get Data from Sir DB Name Sect. Name H B tw tf C	0.11         m           0.11         m           0.01         m           0.01         m
2 Sect Doul Nam Sect Prov the Click	tion ID:7 ble Angle Section ne: Wind Braces, tion : User defined vide the section values as per image < [Apply]	Offset : Center-Center Change Offset Show Calculation Results	Consider Shear Deformation. Consider Warping Effect(7th DOF) OK Cancel	Apply	Offset : Center-Center Change Offset Show Calculation Results	Consider	Shear Deformation. Warping Effect(7th DOF)

Apply

Х

### 3-1. Generation of nodes



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### 3-1.1 Generation of Platform layout (Exterior Column)

#### **Procedure**

Mark the External Columns Using Extrusion

Menu>Elements>Extrude Elements

Select the nodes of the exterior columns

Extrude Elements
 Extrude type: Node- Line
 Element Type: Beam
 Material type: S460
 Section: Exterior Columns
 Translation: Equal Distance
 dx,dy,dz: 0,0,4
 No of times: 1

Click [Apply] > [Close]

	Tree Menu	д
	Node Element Boundary Mass Load	
	1 ude Elements V	
	Start Number	_
	Node Number : 16	
	Element Number : 1	
	Extrude Type	
	Node -> Line Element	
	Source Remove Move	
	Element Attribute	
	Element Type: Beam ~	
	Material :	
	1 1: S460 ~	
	Section :	
	1 1: Exterior column $\checkmark$	
	Beta Angle : 0 V [Deg]	
	2	
	Generation Type	
	Translate Rotate Project	
	Translation	
	Equal Distance	
	O Unequal Distance	
-	dx.dv.dz: 0, 0, 4 m	
	Number of Times : 1	
	Merging Tolerance	
	3 Apply Close	



### 3-2. Generation of Platform layout (Interior Column)



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Menu>Model>Elements>
Extrude Elements
Select the nodes of the Interior column
Extrude Elements Extrude type: Node- Line Element Type Data

Mark the Internal Columns

Using Extrusion

**Procedure** 

Element Type: Beam Material type: S460 Section: Interior Columns Translation: Equal Distance dx,dy,dz: 0,0,4 No of times: 1

Click [Apply] > [Close]



### 3-3.2. Generation of Platform layout (Floor Girders)

Procedure	Node Element Boundary Mass Load
Generating the remaining Floor Girders	Translate Elements
Menu>View>Select>Single	Node Number     31        Element Number     28
Right Click on Model view screen >Elements>Translate	Mode Copy Move Translation Node Increment 0
Translate Elements Mode: Copy Equal Distance: 10.0.0	umber of Times :     1       ● Equal Distance       dx,dy,dz:     10,0,0       Number of Times :     2
Number of Times : 2	Axis: Ox Oy Oz Arbitrary Distances m
Click [Apply] > [Close]	(Example : 5, 3, 4.5 , 3@5.0 ) Direction Vector : 0, 0, 0 m Material Inc. : 0
	Delete Free Nodes         Intersect       Node         Copy Node Attributes          Copy Element Attributes          Merging Tolerance       Apply         Close       Close



# 3-3.3. Generation of Platform layout (Floor Girders)





# 3-3.4. Generation of Platform layout (Floor Girders)

#### Procedure

Generating the remaining Floor Girders

Select the beam as highlighted Right Click on Model view screen Elements>Translate

Translate Elements
 Mode: Copy
 Equal Distance: 0,-8,0
 Number of Times : 3

2 The beams are created.

Click [Apply] > [Close]





### 3-3.5. Generation of Platform layout (Floor Girders)

#### Procedure

Dividing an element Select the beam as highlighted. Right Click on Model view screen Elements>Divide

 Divide Elements
 Element Type : Frame
 Equal Distance
 Number of Divisions x : 4
 (This operation divides the beam into 4 equal segments, with 3 intermediate nodes)

The beam gets divided into 4 parts

Olick [Apply] > [Close]

1	lary Wass Load
Divide Elements	~
Start Number	
Node Number :	34
Element Number :	38
Divide	
Element Type	
O Frame	Solid
⊖ Wall ○	Planar
Equal Distance	
O Unequal Distance	
O Parametric Unequal Di	istance
O Parallel Bracing	
O Divide by Node	
Oivide by Pattern	
Number of Divisions x:	4 🛖
Number of Divisions y:	2
Number of Divisions z:	2
Subdivide Frame Elen	nents
Merce Duplicate Node	es



# 3-3.6. Generation of Platform layout (Floor Girders)

#### Procedure

Generating the remaining Longitudinal Floor Girder

Select the beam as highlighted. Right Click on Model view screen Elements>Extrude

Extrude Elements
 Extrude type: Node- Line
 Element Type: Beam
 Material type: S460
 Section: Floor Girder
 Translation: Equal Distance
 dx,dy,dz: 0,-32,0
 No of times: 1

- The longitudinal girders are created
- 3 Go to intersect elements select all the 3 longitudinal griders and give apply.

Click [Apply] > [Close]







# 4-1 Generation of Wind Braces

× ...

....

....

× ...

En

....

Close

Apply







### 4-1.1. Generation of Remaining Wind Braces

Procedure	Node Element Boundary Mass Loa	d	
Generate remaining Wind Braces	Translate Elements		
Menu>View>Select>Single	Start Number . 52		
Translate Elements	Element Number : 80 .		
	Number of Times : 1	Node Element Boundary Mass Load	
Equal Distance: -10,0,0(for	Equal Distance	Translate Elements V	
X directional bracings)	dx,dy,dz:m	Start Number	
2 0,-32,0 (for Y directional	Number of Times : 2	Node Number : 52	X directional bracings
bracings)	Axis: Ox Oy Oz	Element Number : 80	
Number of Times : 2 All the bracings are generated	Arbitrary           Distances :         m           (Example : 5, 3, 4.5, 3@5.0)	Mode Copy O Move	
Click [Apply] > [Close]	Direction Vector : 0, 0, 0 m Material Inc. : 0 Rep.	Translation       Node Increment       0       Number of Times :       1	
	Section Inc. : 0 - Rep.	Equal Distance     dx,dy,dz: 0,-32,0 m	
	Thickness Inc. : Rep.	Number of Times : 2	V directional bracings
	Intersect Node Elem	O Unequal Distance	
	Copy Node Attributes	Axis: 🔘 x 🔾 y 📿 z	
	Copy Element Attributes	Arbitrary	
		Distances : m	
	Apply Close	(Example : 5, 3, 4.5, 3(05.0)) Direction Vector :	
		0, 0, 0 m	

### 5-1. Generation of Storey

#### **Procedure**

Generate remaining Storey's by Translation

Menu>View>Select>Select All
 Right Click, Elements>Translate

2 Translate Elements Mode: Copy

Equal Distance: 0,0,4

Number of Times : 1

3 The entire structure is created. Click [Apply] > [Close]

		Gen			
View Structure Node/Element Pro	operties Bour Load Analysis	Results Pushover Design	Seismic Performance Query Tools	1	
Redraw Initial Previous Danamic View Point V	Arender View	Ctrl+Shift+S	Crids/Snap Display Window	Close * Tile Horizontally Next II Tile Vertically Previous Cascade W Window Tile	
Node Element Boundary Mass Load	Intersect Line				1.0
Translate Elements	🛱 Plane 🕅 Volume	Ctrl+Shift+A			
Start Number       Node Number       Element Number       80				$\square$	
Mode Copy Move Translation Node Increment 0	Nodes Eements Properties Boundaries Different Create Lin Create Create Lin Create Create Lin Create Create Create Lin Create Create C	ments Alt+1 e Eements on Curve nverted Line Elements		X	A
Aumber of Times :     1       Equal Distance       dx,dy,dz:     0, 0, 4       Number of Times :     1	Masses Dates Load Dates Modify Node Modify Element Activities X Divide	Ait+2 . Ait+3 Ait+4 Ait+5 Ait+6 Ait+6 Ait+7			
Axis: x y z Arbitrary	QC Zoom 2 <sup>23</sup> Merge Q <sup>+</sup> Pan X Intersect. P Rotate 2 <sup>24</sup> Compact Cancel 1 <sup>25</sup> Renumbe Cancel 1 <sup>25</sup> Renumbe	Alt+8 ement Parameters Alt+9 Numbers Alt+0 ing			
(Example : 5, 3, 4.5, 3@5.0) Direction Vector : 0, 0, 0 m	SB Elements Modify El SB Element I	Table Ctrl+Alt+M			
Section Inc.     :     0     Rep.       Thickness Inc.     :     0     Rep.       Delete Free Nodes					
Intersect Node Elem Copy Node Attributes Copy Element Attributes Merging Tolerance					
3 Apply Close			·		

### 5-2. Generation of Platform layout (Columns)



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Generate Roof Truss

- Structure>Wizard>Base structures>Truss.
- 2 From the input tab, enter the parameters as shown in the figure
- From the Edit tab, enter the verticals, material and section type as shown in the figure
- From the insert tab, use node 1 of the truss to coincide with node 68 of the frame model.
- 5 Click [Apply] > [Close]



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### 6-2. Generation of Vertical members of the Truss (1)







# 6-4. Generation of Trusses (1)

Close

Apply

Procedure	Node Element Boundary Mass Load
Generating the remaining Truss	Translate Elements
Menu>View>Select Previous Right Click, Elements>Translate	Start Number       Node Number       Element Number       220
Translate Elements	Mode Copy O Move
Mode: Copy	Translation
Equal Distance: 0,4,0	1 Number of Times : 1
Number of Times : 8	Equal Distance dx,dy,dz: 0, 4, 0 m
The entire roof truss is created.	O Unequal Distance Axis: O x O y O z O Arbitrary
	Distances : m (Example : 5, 3, 4,5 , 3@5.0 ) Direction Vector : 0, 0, 0
	Material Inc. : 0 🗣 🗌 Rep.
	Section Inc. : 0 🗬 🕞 Rep.
	Thickness Inc. : 0 Rep.
	Intersect Node Elem
	Copy Node Attributes
	Merging Tolerance



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# 6-5. Generation of Trusses (2)



# 6-5. Generation of Trusses (2)





### 6-6. Generation of Roof Bracing (1)



## 6-7. Generation of Roof Bracing (2)



# 6-8. Generation of Roof Bracing (3)



# 7-1. Generation of Fixed Supports

#### Procedure

Generating the supports at base Menu>View>View Point>Front Menu>View>Select>single Menu>Model>Boundaries>Supports

#### 1 Supports

Option: Add D-All: Checked in R-All: Checked in

Fixed supports are generated.

Click [Apply] > [Close]

![](_page_32_Figure_8.jpeg)

![](_page_32_Figure_9.jpeg)

#### **Procedure**

Creating Static Load cases

Menu>Load> Static Load Cases

Name: Self Weight Type: Dead Load

> Define the remaining loads according to their Specifications as shown in the dialog box

Ty De	pe scriptio	: Dead Load ( on : Load due to	D) its own weight		Modify Delete
ĵ	No	Name	Туре	Description	
•	1	Self weight	Dead Load (D)	Load due to its own weight	
Ĵ	2	LL	Live Load (L)	variable weight on a structu	
_	3	wind load X	Wind Load on Structur	weight due to wind on a str	
	4	wind load Y	Wind Load on Structur	weight due to wind on a str	
	5	Roof wind load	Wind Load on Structur	weight due to wind on the r	
*					

### 8-2. Applying the Self Weight

![](_page_34_Figure_2.jpeg)

![](_page_34_Figure_3.jpeg)

![](_page_35_Picture_0.jpeg)

### 8-3. Applying the Live Load

![](_page_35_Figure_2.jpeg)

x1 = 0 ; x2 = 1 ; w = -11.5KN/m

(On the outer girder we apply udl of -5.6KN/m and on the inner girders the udl is of -11.5 KN/m) Click [Apply] > [Close]

				~	
Load	Group Na	me			
Defa	ult			~	Ŀ
Optio	ns				
OA	dd 🔾	Repla	ace	⊖ De	elete
Load	Туре				
Unif	orm Load	s			
Nı	×.	Ж2		N2	
		<u>X2</u>		N2	
Dire	ccentricity	X2	Global	N2	
Dire Proj	ccentricity ction :	<u>X2</u>	Global Z	N 2	D No
Dire Proj	ccentricity ection :	X2	Global Z Yes	N 2	) No
Dire Proj Value	ccentricity cction : jection :	<u>X2</u>	Global 2 O Yes O Abso	N2	) No
N14	ccentricity ection : pection : Relative	W2	Global 2 Yes Abso	N 2	No
N14 Dire Pro Value X1 x2	ccentricity ection : jection : Relative 0 1	X2	Global 2 Yes Abso -11.5 0	N2	No
N10 Dire Proj Value x1 x2 x3	ccentricity ection : ection : Relative 0 1 0	<u>X2</u>	Global 2 Yes Abso -11.5 0 0	N2 Z	D No

![](_page_35_Figure_6.jpeg)

![](_page_36_Picture_0.jpeg)

# 8-4. Applying the Wind Load (Wind X +)

#### Procedure

Applying an uniform distributed load as wind load on the X direction outer columns of the structure in load case 3: Wind X +

Right Click, Loads> Element Beam Load

### Element Beam Load Load Case Name : Wind X + Load Group Name: Default Options: Add Load Type: Uniform Loads Direction : Local Z Value : Relative x1 = 0 ; x2 = 1 ; w = 9.8KN/m

(An udl of 4.9KN/m, 9.8KN/m, 2.7KN/m and 5.5KN/m has been applied on the columns)

Click [Apply] > [Close]

![](_page_36_Figure_8.jpeg)

![](_page_36_Figure_9.jpeg)

# Step **08**

# 8-5. Applying the Wind Load (Wind X -, Y+ and Y-)

#### Procedure

- Applying an uniform distributed load as wind load on the X and Y direction outer columns of the structure in load case 4, 5 and 6: Wind X - ,Y + and Y -
- Similarly, we generate the wind load as udl on the other directions (X -, Y + and Y -) and on the roof truss

Load	Case Na	me				
wind	l load Y			$\sim$		
Load	Group Na	ame				
Defa	ult			~		
Optic	ons					
0/	Add C	Repla	ice	ODe	lete	
Load	Туре					
Uni	form Load	ls			~	
Nı	 ≍-↓	X2		N2		
N1		X2		N2		
Dire	ccentricity ection :	X2	Global	N2 Y	~	
Dire Pro	ccentricity ection :	X2	Global Yes	N2	No	
N1	ccentricity ection : jection :	X2	Global Yes	Y V	No	
Dirv Pro Value	ccentricity ection : pjection : Relative	X2	Global Yes Abso 6.1	Y V	No	
N1	cccentricity ection : Relative 0	X2	Global Yes Abso 6.1 0	Y Y	) No	
N1 N	cccentricity ection : fection : Relative	¥2	Global Yes Abso 6.1 0 0	Y Y	) No	

	Element	DO	undary	IVIdSS	Lo
Elemer	nt Beam Lo	ads			
Load	Case Nam	ne			
Roof	wind load			× .	
Load	Group Nar	me			
Defa	ult			× .	
Optio	ons				
04	Add 🔘	Repla	ce (	Delete	e
Load	Туре				
Unit	form Loads	5			$\sim$
Nı		X2	↓ • N	12	
	ccentricity	X2		12	
Dire	ccentricity	X2	Local z	12	~
Dire Pro	ccentricity ection : jection :	X2	Local z	0 N	~
Dire Value	ccentricity ection : jection :	X2	Local z Yes	0 N	√ io
Dire Pro Value	ccentricity ection : jection : e Relative	¥2	Local z Yes	0 N ute	~ 0
Dire Pro Value	ccentricity ection : jection : Relative 0	X2	Local z Yes Absol	0 N ute	~
E Dird Pro Value x1 x2	ccentricity ection : jection : Relative 0 1	X2	Local z Yes Absol	O N ute	~
N11	ccentricity ection : jection : e Relative 0 1 0	X2	Local z Yes Absol 5.4 0	Nute	~ 0
N11	ccentricity ection : pection : e Relative 0 1 0 0	X2	Local z     Yes     Absol     5.4     0     0     0	O N ute	~
N11 Dire Pro Value X1 x2 x3 x4	ccentricity ection : jection : Relative 0 1 0 0	X2 ( ( W	Local z     Yes     Absol     5.4     0     0     : kN/m	O N ute	•

	Load	Description	Intensity
	Dead Load	Self Weight	
	Live Load	Outer Girder Inner Girder	11.5 kN/m 5.6 kN/m
	Roof Load	5.4 kN/m	
	Wind Load	Wind X (inner) Wind X ( outer) Wind Y (inner) Wind Y(outer)	4.9 kN/m 9.8 kN/m 2.7 kN/m 5.5 kN/m
	94 94 94 94 94		
A A A SA			

#### Procedure

Performing Analysis

Menu>Analysis>Perform Analysis

- The short-cut is available on the Tool Bar.
  - a. Perform Analysis
  - b. Preprocessor
  - c. Postprocessor

Click Analysis in the Icon Menu or select **Analysis>Perform Analysis** in the Main Menu to analyze the model. Once the analysis is completed, the program switches automatically to the **post-processing** mode, which provides access to the analysis and design results.

Click **Preprocessing Mode** in the Icon Menu or select **Mode> Preprocessing Mode** in the Main Menu when the preprocessing mode has to be restored to modify the data.

![](_page_38_Picture_11.jpeg)

Note: Remove beam end releases before performing analysis

#### Procedure

Assigning the Code of practice for load combination Generation Menu>Results > Load Combination > Steel Design> Auto Generation

 Click on Select Design Code Eurocode3:05

Click [OK] > [Close]

![](_page_39_Picture_6.jpeg)

Set Load Cases and Direction...

OK

Cancel

	No	Name	Active	Type	Description		LoadCase	Factor	
•	1	sLCB1	Streng	Add	1.35D + 1.5(1.0LL)	•	Self weight(ST)	1.3500	
	2	sLCB2	Streng	Add	1.35D + 1.5(1.0LL)		LL(ST)	1.5000	
	3	sLCB3	Streng	Add	1.35D + 1.5(1.0LL)	*			
	4	sLCB4	Streng	Add	1.35D + 1.5(1.0LL)		-		
	5	sLCB5	Streng	Add	1.35D + 1.5(0.7LL)				
	6	sLCB6	Streng	Add	1.35D + 1.5(0.7LL)				
	7	sLCB7	Streng	Add	1.35D + 1.5(0.7LL)				
	8	sLCB8	Streng	Add	1.35D + 1.5(1.0LL)				
	9	sLCB9	Streng	Add	1.35D + 1.5(1.0LL)				
	10	sLCB1	Streng	Add	1.35D + 1.5(1.0LL)				
	11	sLCB1	Streng	Add	1.35D + 1.5(0.7LL)				
	12	sLCB1	Streng	Add	1.35D + 1.5(0.7LL)				
	13	sLCB1	Streng	Add	1.35D + 1.5(0.7LL)				
	14	sLCB1	Servic	Add	SERV :1.0D + (1.0				
	15	sLCB1	Servic	Add	SERV :1.0D + (1.0				
$\rightarrow$	16	sLCB1	Servic	Add	SERV :1.0D + (1.0				
-	17	sLCB1	Servic	Add	SERV :1.0D + (1.0				
-	18	sLCB1	Servic	Add	SERV :1.0D + (1.0				
-	19	sLCB1	Servic	Add	SERV :1.0D + (1.0				
-	20	sLCB2	Servic	Add	SERV :1.0D + (1.0				
+	21	sLCB2	Servic	Add	SERV :1.0D + (0.7				
	22	sl CB2	Servic	Add	SERV 1 0D + (0 7				

\* The load combinations, in conformity with the *Limit State Design Method*, are auto-generated by Auto Generation. Additional Service Load combinations may be entered to examine displacements and reactions

# 10-2. Verification of Reactions (1)

![](_page_40_Figure_2.jpeg)

# 10-3. Displacement contour (2)

#### Procedure

Menu>Results >Deformations> Displacement Contour

- Load Cases/Combinations CBS:sLCB10
- Click on values for displaying, for Animation check Animate

![](_page_41_Picture_6.jpeg)

![](_page_41_Picture_7.jpeg)

# 10-4. Verification of Member (3)

![](_page_42_Figure_2.jpeg)

![](_page_42_Figure_3.jpeg)

# 11-1. Design (1)

#### **Procedure**

Assigning the Code of practice for designing the structure

Menu>Design>> Steel Design Parameter

Design Code > Eurocode3:05

Design Code :	Eurocode3:05	~
National Annex :	Recommended	~
All Beams/Gird	lers are Laterally Brac	ed
Check Beam/C	olumn Deflection	
Apply Special	Provisions for Seismic	Design
iaxial moments fo	or buckling resistance	
Biaxial moment	ts at the same locatio	n
Maximum mor	nents along the meml	ber
Consider as lir (Eq.6.2) Nethod for interac	iear summation for cl	ass 1,2
O By Code	O Annex A	O Annex B
oint of load applie	cation for Mcr(Beam)	
Отор	O Shear Center	OBottom

### 11-2. Design (a)

#### **Procedure**

Menu>Design> Steel Code Check

Member (on) Click on Graphic

Select the structural element you would like to view the graphic detailing of

Code : E Sorted b	:C3:05 y	Member		U	nit: kN	, n
Sorted b	w	Member				
		Property		Change	Update	
CUIK	MEMB	SECT	CEL	Section	n	
CHK	COM	SHR	SEL	Material	Fy	1
01	1	1	_	Exterior column	HEA400	
OK	0.081	0.018		S460	460000	
OK	2	1	_	Exterior column	HEA400	
OK	0.068	0.008		S460	460000	
ov	3	1	_	Exterior column	HEA400	
	0.058	0.011	1 1 1	S460	460000	
ov	4	1	-	Exterior column	HEA400	
OK -	0.269	0.068		S460	460000	
OK	5	1	_	Exterior column	HEA400	
	0.131	0.036		S460	460000	
ov	6	1	_	Exterior column	HEA400	
OK	0.249	0.068		S460	460000	
ov	7	1	_	Exterior column	HEA400	1
OK	0.135	0.037	1 1 1	S460	460000	
ov	8	1	_	Exterior column	HEA400	
OK	0.269	0.068		S460	460000	
01/	9	1	-	Exterior column	HEA400	
	0.131	0.036	1 1 1	S460	460000	
or	10	1	_	Exterior column	HEA400	
ON	0.071	0.018		S460	460000	
OK	11	1		Exterior column	HEA400	
OK	0.068	0.008	L	S460	460000	
OK	12	1	F	Exterior column	HEA400	
ON	0.043	0.011		S460	460000	
OK	13	2		Interior column,	HEA500	
	0.220	0.045		S460	460000	
Conn	ect Mode	l View		View Resu	lt Ratio	
Sele	ct All	Unsele	ct All	Re-calculat	ion	>>
Grap	hic	Detai	il	Summary	Clos	e

Eurocode3:05 Code Checking Result Dialog

Į	Preview Wind	low		_		×
ľ	Memb No : 4	V 🞒 Print 🎒 Print All 📳 Close 日	Save			
1	. Design Inform	nation	z			
	Design Code	Eurocode3:05				- 1
	Unit System	kN, m	Ŭ			- 1
	Member No	4	8			- 1
	Material	S460 (No:1)	0			- 1
		(Fy = 460000, Es = 210000000)	6 0.0°			- 1
	Section Name	Exterior column (No:1)				- 1
		(Rolled : HEA400).	+			- 1
	Member Length	: 4.00000	0.3			- 1
						- 1
2	Member Ford	ces				- I
	Avial Force	Evy = -786.48 (LCB: 10 POS: I)	Depth 0.39000 Top F Width 0.30000	Top F Thick 0.01900		- 1
	Bending Moments	My = 150.651 Mz = -10.530	Bot.# Width 0.30000	Bot.F Thick 0.01900		. II
	End Moments	Code Europosition term kN, m rNo 4 i S400 (No:1) (F) = 460000, Es = 210000000) Name Esterior column (No:1) (Rolled : HEA400). rLength : 400000 Por Forces ments $M_1 = -115.82, M_2 = 159.651, Mz = -10.339$ ments $M_1 = -115.82, M_2 = 159.651 (for Lb)$ $M_1 = -115.82, M_2 = 159.651 (for L)$ $M_1 = -115.82, M_2 = 159.651 (for L)$ $M_2 = -3.6407, Mz = -10.539 (for L)$ $M_2 = -3.03.20 (LCB: 11, POS.1)$ <b>n</b> Parameters ed Lengths $Ly = 4.00000, Lz = 4.00000, Lb = 4.00000 = Length Footors Ky = 1.00, Kz = 1.00ent Uniform Moment Factors Cmy = 0.85, CmL T = 1.00king Resultmess Ratio\gamma = 545.2000 (LCB: 1)Mz = 19.85/1178.52 = 0.135 < 1.000Mz = 12.020/140/1479/140)Mz = (Mz = My/L, My - Rd) + (Mz = Mz = Rd) [Mz = Mz $				
	End Woments	My1 = -115.02, My1 = 150.651 (for Ly)	W 0.00045	taz 0.00009		- 1
		Mai = 5 26407 Mai = -10 530 (for La)	Yber 0.15000 Wely 0.00231	Zber 0.19500 Welz 0.00057		- 1
	Shear Former	En = 4.64784 (LCR: 0. POSI)	ry 0.16800	rz 0.07340		
	onear Porces	Fyy = 4.04761 (LCB: 44 BOSt)				- 1
		P22 = -103.20 (LCB: 11, P03.1)				- 1
3	Design Parar	neters				- 1
	Lieberger Lieberger		1 - 4 00000			- 1
	Effective Length Fr	$E_y = 4.00000, E_z = 4.00000,$	20 - 4.00000			- 1
	Effective Length Pa	Actors Ry = 1.00, KZ = 1.00	T- 100			- 1
	Equivalent Oniom	n Moment Padors City = 0.85, Citiz = 0.85, Citi	1.00			- 1
4	Checking Re	sult				- 1
	Slenderness Batio					- 1
	KL/r = 54.5	5 < 200.0 (LCB: 1)				- 1
	Axial Resistance					- 1
	N Ed/MININo	Rd. Nb. Rdl = 786.48/6197.44 = 0.127 < 1.000	ок			- 1
	Bending Resistance					- 1
	M Edv/M Bd	v = 159.65/1178.52 = 0.135 < 1.000	ж			- 1
	M Edz/M Rd	z = 10.539/398.198 = 0.026 < 1.000	ок			- 1
	Combined Resista	ince				- 1
	R.MNRd = MA	XIM Edv/Mnv Rd M Edz/Mnz Rd 1				- 1
	R.BiM = (M.E	dy/Mny_Rd)^a + (M_Edz/Mnz_Rd)^β				- 1
	R.byN = N_E	d/(A*fv/vM0), R.bvM = M_Edv/Mv_Rd + M_Edz/Mz_Rd				- 1
	RoLT1 = N E	d/(xy*A*fv/vM1)				- 1
	Rb.LT1 = (kyy	*M_Edv)/(xLT*Wplv*fv/vM1) + (kvz*M_Edz)/(Wplz*fv/vM1)				- 1
	RoLT2 = N_E	d/(xz*A*fv/vM1)				- 1
	Rb.LT2 = (Kzy	*M. Edv)/(vLT*Wplv*fv/vM1) + (Kzz*M. Edz)/(Wplz*fv/vM1)	<pre></pre>			
	Rmax = MAX	E MNRd, R BiM (R byN+R byM), MAX(Rol T1+Rb L T1, F	RoLT2+RbLT2)1= 0.269	1.000 O.K		
	Shear Resistance					
	V_Edv/Vv_Rd	= 0.001 < 1.000 OK				
	V_Edz/Vz_Rd	= 0.068 < 1.000				
5	Deflection Ch	necking Results				
	L/ 300.0 = 0.0133	> 0.0043 (Memb:4, LCB: 24, Dir-X)	ж			
					_	

### 11-2. Design (b)

#### **Procedure**

Menu>Design> Steel Code Check

#### Click on Detail

Select the structural element you would like to check

A window explaining the design logic of the structural element appears with detailed relevance to the IS Code

code :	EC3:05			Ur	nit: kN	, n
orted	by 📍	Member Property		Change	Update	·
CUK	MEMB	SECT	CE1	Section	n	
CHK	COM	SHR		Material	Fy	11
014	1	1	_	Exterior column	HEA400	
UK	0.081	0.018	1 1 1	S460	460000	
ov	2	1	_	Exterior column	HEA400	
OK	0.068	0.008		S460	460000	ste
014	3	1	_	Exterior column	HEA400	
OK	0.058	0.011	1 1 1	S460	460000	
014	4	1	_	Exterior column,	HEA400	
OK	0.269	0.068		S460	460000	
	5	1	-	Exterior column	HEA400	
OK	0.131	0.036	1 1 1	S460	terior column, HEA400 S460 460000 terior column, HEA400 S460 460000 terior column, HEA400	
-	6	1	-	Exterior column	HEA400	
OK	0.249	0.068		S460	460000	
-	7	1	_	Exterior column	HEA400	)
ок 0 ок 0 ок 0 ок 0	0.135	0.037		S460	460000	
	8	1	_	Exterior column	HEA400	
OK	0.269	0.068		S460	460000	
-	9	1	-	Exterior column	HEA400	
OK	0.131	0.036		S460	Update on Fy in, HEA400 460000 in, HEA400 in, HEA500 in, HEA5	
	10	1	-	Exterior column	HEA400	
ок ок ок ок ок	0.071	0.018		S460	460000	
014	11	1	_	Exterior column	HEA400	
OK	0.068	0.008	1 1 1	S460	460000	
014	12	1	-	Exterior column	HEA400	
OK	0.043	0.011		S460	460000	
01/	13	2	_	Interior column,	HEA500	
OK	0.131         0.036         S460         460000           6         1         Exterior column, HEA400           0.249         0.068         F         S460         460000           7         1         Exterior column, HEA400         S460         460000           0.135         0.037         F         Exterior column, HEA400         S460         460000           8         1         F         Exterior column, HEA400         S460         460000           9         1         F         Exterior column, HEA400         S460         460000           10         1         F         Exterior column, HEA400         S460         460000           11         1         F         Exterior column, HEA400         S460         460000           11         1         F         Exterior column, HEA400         S460         460000           12         1         F         Exterior column, HEA400         S460         460000           13         2         1         F         Exterior column, HEA400         S460         460000           13         2         1         F         Exterior column, HEA400         S460         460000           0.220 <t< td=""></t<>					
Con	nect Mode	l View		View Resu	lt Ratio	
Sel	ect All	Unsele	ct All	Re-calculati	ion	>>
Gra	phic	Deta	il	Summary	Clos	e

midas Gen - Steel Code Checking [ Eurocode3:05 ] Gen 2025
+=====================================
<pre>Steel Member Applicable Code Checking Based On Eurocode3:05, Eurocode3, AISC(15th)-LRFD16, AISC(15th)-ASD16, AISC(14th)-LRFD05, AISC(14th)-ASD10, AISC(14th)-LRFD05, AISC(14th)-ASD05, AISC-LAFD24, AISC-RFD03, AISC-ASD08, CAS-16-01, BS3950-24, BS3950-90, AI3-ASD08, CAS-16-01, BS3950-24, BS3950-20, IS:000-1984, KOS (41.30 : 2022, KOS 41.31 : 2019, KSSC-LSD06, AISC-ASD03, TNM-ASD06, TNM-LSD07, KSC-ASD06, AIR-ASD03, TNM-ASD06, TNM-LSD07, TNM-ASD09, TNM-LSD09, KSCP 2015(LRF0), NSCP 2015(ASD), SP 16.13330.2017</pre>
(c)SINCE 1989
MIDAS Information Technology Co.,Ltd. (MIDAS IT) MIDAS IT Design Development Team
HomePage : www.MidasUser.com
+========+   Gen 2025
+=======+
*. DEFINITION OF LOAD COMBINATIONS WITH SCALING UP FACTORS.
LCB C Loadcase Name(Factor) + Loadcase Name(Factor) + Loadcase Name(Factor)
1 1 Self weight(1.350) + LL(1.500) + wind load X(0.900) 3 1 Self weight(3.550) + LL(1.500) + wind load Y(0.900) 4 1 Self weight(3.550) + LL(1.500) + word load Y(0.900) 5 1 Self weight(3.550) + LL(1.500) + Roof wind load (1.900) 6 1 Self weight(3.550) + LL(1.050) + wind load X(1.500) 6 1 Self weight(3.550) + LL(1.050) + wind load (1.500) 7 1 Self weight(3.550) + LL(1.050) + wind load (1.500) 8 1 Self weight(3.550) + LL(1.050) + wind load (1.500) 8 1 Self weight(3.550) + LL(1.050) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.550) + Wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + Wind load X(-0.900) 9 1 Self weight(3.550) + LL(1.1500) + Wind load X(-0.900) 9 1 Self weight(3.550) + Wind load X
10 1 Self weight (1.350) + LL (1.500) +Roof wind load (-0.900)

5 1		
	Self weight( 1.350) +	LL( 1.050) + wind load X( 1.500)
6 1	Self weight( 1.350) +	LL( 1.050) + wind load Y( 1.500)
7 1	Self weight( 1.350) +	LL( 1.050) +Roof wind load( 1.500)
8 1	Self weight( 1.350) +	LL( 1.500) + wind load X(-0.900)
9 1	Self weight( 1.350) +	LL( 1.500) + wind load V(-0.900)
10 1	Self weight( 1 350) +	<pre>LL( 1.500) +Poof wind load(-0.900)</pre>
11 1	Self weight( 1 350) +	11 (1.850) + wind load X(-1.500)
12 1	Salf weight( 1 350) +	LL(1.050) + wind load X(-1.500)
13 1	Salf weight( 1.350) +	LL( 1.050) + Roof wind load(-1.500)
14 2	Self weight( 1.000) +	11(1.000) (1001 1010 1010(-1.500)
15 2	Self weight ( 1.000) +	U(1.000) + wind load Y(0.690)
16 2	Self weight( 1.000) +	LL(1.000) + wind load X(0.000)
17 2	Self weight( 1.000) +	LL(1.000) + Wind load (0.600)
18 2	Self weight( 1.000) +	LL(1.000) + wind land Y(-0.600)
10 2	Self weight( 1.000) +	LL(1.000) + wind load X(-0.000)
20 2	Self weight( 1.000) +	LL( 1.000) + Wind 1010 (-0.000)
20 2	Self weight( 1.000) +	LL( 0.700) + wind load (-0.000)
21 2	Self weight( 1.000) +	LL(0.700) + wind 1040 X(1.000)
22 2	Self weight( 1.000) +	LL(0.700) + Wind 1040 ((1.000)
23 2	Self weight( 1.000) +	LL( 0.700) + wind 1020( 1.000)
das Gen - S	teel Code Checking [ Eurocode	3:05 1 Gen 2025
das Gen - S	teel Code Checking [ Eurocode	3:05 ] Gen 2025
das Gen - S	teel Code Checking [ Eurocode	-3:05 ] Gen 2025
das Gen - S 25 2	<pre>Self weight( 1 000) +</pre>	.3:05 ] Gen 2025
das Gen - S 25 2 26 2	<pre>self weight( 1.000) + Self weight( 1.000) +</pre>	E3:05 ] Gen 2025
25 2 26 2 27 2	<pre>teel Code Checking [ Eurocode Self weight( 1.000) + Self weight( 1.000) + Self weight( 1.000) +</pre>	LL(0.700) + wind load Y(-1.000) LL(0.700) + koof wind load(-1.000) LL(0.700) +Roof wind load(-1.000)
25 2 26 2 27 2 28 2	<pre>self weight( 1.000) + Self weight( 1.000) + Self weight( 1.000) + Self weight( 1.000) + Self weight( 1.000) +</pre>	LL(0.700) + wind load Y(-1.000) LL(0.700) + kind load Y(-1.000) LL(0.700) +Roof wind load (-1.000) LL(0.500) LL(0.200) + wind load X(0.200)
25 2 26 2 27 2 28 2 29 2	<pre>self weight( 1.000) + Self weight( 1.000) +</pre>	L1(0.700) + wind load Y(-1.000) L1(0.700) + Roof wind load (-1.000) L1(0.500) + wind load X(0.200) L1(0.300) + wind load X(0.200) L1(0.300) + wind load X(0.200)
25 2 26 2 27 2 28 2 29 2 30 2	Self weight (1.000) + Self weight (1.000) +	L1(0.700) + wind load Y(-1.000) L1(0.700) + koof wind load (-1.000) L1(0.700) +Roof wind load (-1.000) L1(0.500) + wind load X(0.200) L1(0.300) + wind load X(0.200) L1(0.300) + wind load Y(0.200)
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2	<pre>teel Code Checking [ Eurocode Self weight( 1.000) + Self weight( 1.000) +</pre>	L1 (0.700) + wind load Y(-1.000) L1 (0.700) + Moof wind load (-1.000) L1 (0.700) + Moof wind load (-1.000) L1 (0.500) + wind load X (0.200) L1 (0.300) + wind load X (0.200) L1 (0.300) + wind load (2.200) L1 (0.300) + Moof wind load (2.200)
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2	<pre>Self weight (1.000) + Self weight (1.000) +</pre>	L1(0.700) + wind load Y(-1.000) L1(0.700) + %oof wind load (-1.000) L1(0.700) +%oof wind load (-1.000) L1(0.500) + wind load X(0.200) L1(0.500) + wind load X(0.200) L1(0.500) + wind load X(-0.200) L1(0.500) + wind load X(-0.200) L1(0.500) + wind load Y(-0.200)
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 32 2 32 2	<pre>teel Code Checking [ Eurocode</pre>	<pre>13:05 ] Gen 2025 LL( 0.700) + wind load Y(-1.000) LL( 0.700) +Roof wind load (-1.000) LL( 0.500) LL( 0.500) + wind load X( 0.200) LL( 0.300) + wind load X( 0.200) LL( 0.300) + wind load X( 0.200) LL( 0.300) + wind load X(-0.200) + wind load X(-0.200) LL( 0.300) + wind load X(-0.200) + wind</pre>
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 33 2 34 2	<pre>teel Code Checking [ Eurocode Self weight( 1.000) + Self weight( 1.000) +</pre>	13:05 ]         Gen 2025           LL( 0.700) + wind load Y(-1.000)         LL( 0.700) + Moor wind load (-1.000)           LL( 0.500)         LL( 0.500)           LL( 0.300) + wind load X( 0.200)         LL( 0.300) + wind load X( 0.200)           LL( 0.300) + wind load X( 0.200)         LL( 0.300) + wind load X( 0.200)           LL( 0.300) + wind load X( -0.200)         LL( 0.300) + wind load X( -0.200)           LL( 0.300) + wind load X( -0.200)         LL( 0.300) + wind load X( -0.200)
25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 34 2	<pre>teel Code Checking [ Eurocode</pre>	L1 (0.700) + wind load Y(-1.000) L1 (0.700) + koof wind load (-1.000) L1 (0.700) + koof wind load (-1.000) L1 (0.300) + wind load X (0.200) L1 (0.300) + wind load Y (-0.200) L1 (0.300) + wind load Y(-0.200) L1 (0.300) + wind load Y(-0.200) L1 (0.300) + wind load (-0.200) L1 (0.300) + wind load (-0.200) + wind load (-0.200) L1 (0.300) + wind load (-0.200) + wind load (-0.20
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 34 2	<pre>teel Code Checking [ Eurocode</pre>	13:05 ]         Gen 2025           LL( 0.700) + wind load V(-1.000)         LL( 0.700) +Roof wind load (-1.000)           LL( 0.500) + wind load X( 0.200)         LL( 0.300) + wind load X( 0.200)           LL( 0.300) + wind load X(-0.200)         LL( 0.300) + wind load X(-0.200)           LL( 0.300) + wind load X(-0.200)         LL( 0.300) + wind load Y(-0.200)           LL( 0.300) + wind load Y(-0.200)         LL( 0.300) + wind load Y(-0.200)           LL( 0.300) + wind load Y(-0.200)         LL( 0.300) + wind load Y(-0.200)           LL( 0.300) + wind load Y(-0.200)         LL( 0.300) + wind load Y(-0.200)
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 34 2	<pre>teel Code Checking [ Eurocode Self weight( 1.000) + Self weight( 1.000) +</pre>	L1 (0.700) + wind load Y(-1.000) L1 (0.700) + wind load Y(-1.000) L1 (0.700) + Roof wind load (-1.000) L1 (0.500) + wind load X (0.200) L1 (0.300) + wind load X (0.200) L1 (0.300) + wind load X (-0.200) L1 (0.300) + wind load Y(-0.200) L1 (0.300)
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 33 2 34 2 	<pre>teel Code Checkking [ Eurocode Self weight( 1.000) + Self wei</pre>	L1 (8.788) + wind load Y(-1.808) L1 (8.788) + Roof wind load (-1.808) L1 (8.788) + Roof wind load (-1.808) L1 (8.386) + wind load X (8.208) L1 (8.386) + wind load X (8.208) L1 (8.386) + wind load X (-2.208) L1 (8.386) + wind load X (-2.208) L1 (8.386) + wind load Y(-2.208) L1 (8.386) + wind load Y(-2.208) H1 (8.386) + wind load Y(-2.20
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 34 2 34 2 34 2	<pre>teel Code Checking [ Eurocode Self weight( 1.000) + Self weig</pre>	13:05 ]     Gen 2025       LL( 0.700 + wind load Y(-1.000)       LL( 0.700 + Moor wind load Y(-1.000)       LL( 0.500 +       LL( 0.300 + wind load X( 0.200)
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 34 2 das Gen - S	<pre>teel Code Checking [ Eurocode Self weight( 1.000) + Self weight( 1.000) +</pre>	13:05 ]     Gen 2025       LL(0.700) + wind load Y(-1.000)       LL(0.700) + Roof wind load (-1.000)       LL(0.500)       LL(0.300) + wind load X(0.200)       LL(0.300) + wind load X(0.200)       LL(0.300) + wind load X(0.200)       LL(0.300) + wind load X(-2.200)       LL(0.300) + wind load X(-0.200)       LL(0.300) + wind load X(-0.200)       LL(0.300) + wind load Y(-0.200)
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 34 2 34 2 das Gen - S	<pre>teel Code Checking [ Eurocode Self weight( 1.000) + Self weight( 1.000) +</pre>	13:05 ]     Gen 2025       LL( 0.700 + wind load Y(-1.000)       LL( 0.700 + Moof wind load (-1.000)       LL( 0.500 +       LL( 0.300 + wind load X( 0.200)
das Gen - S 25 2 26 2 27 2 28 2 29 2 30 2 31 2 32 2 33 2 34 2 	<pre>teel Code Checking [ Eurocode</pre>	13:05 ]         Gen 2025           LL(0.700) + wind load V(-1.000)         LL(0.700) + Roof wind load (-1.000)           LL(0.500)         LL(0.300) + wind load X(0.200)           LL(0.300) + wind load X(0.200)         LL(0.300) + wind load X(0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + wind load Y(-0.200)         LL(0.300) + wind load Y(-0.200)           LL(0.300) + Wind load Y(-0.200)         LL(0.300)

\*. LOADCOMB NO = 2, MATERIAL NO = 1, SECTION NO = 1 \*. UNIT SYSTEM : kN, m

# 11-3. Design (optimising design)

#### Procedure

Menu>Design> Steel Code Check

- Select the structural element you would like to optimize, click on change
- Click on Search Satisfied Section. Make sure the Limit Combined ratio is as per your preference.
  - Such a dialog box with the available optimum options appears

	EC3:05			U	nit:kN ,	
orted	by O	Member Property		Change	Update.	
CUIK	MEMB	SECT	0.51	Section		
CHK	COM	SHR	SEL	Material	Fy	
014	87	1	_	Exterior column	, HEA400	
OK	0.469	0.142	L	S460	460000	
ov	94	2		Interior column,	HEA500	
UK	0.329	0.091		S460	460000	
NG	34	3		Floor girders, I	HEA260	
	1.352	0.314		S460	460000	
OK*	699	5	F	Roof Brac	ing	
	0.941	0.004		S460	460000	
NC	386	6	_	Diagonal me	mbers	
NG	1.361	0.038		S460	460000	
OK	158	7	_	Wind brac	ces	
ON	0.753	0.000		S460	460000	

Connect Mode	View	View Result F	Ratio
Select All Unselect All		Re-calculation	>>
Graphic	Detail	Summary	Close

Once MCD File							_	_	
Open MGB File	_		Fron	n To	_		From	m To	
INIT ST	Same	e H	0	0	s	ame tw	0	0	
	Same	Same B1		0	s	ame tf1 0	0	0	
I ~	Same	e B2	0	0	s	Same tf2	0	0	
Limit Combined Rat	o from 0.8		to 1		Search Sat		tisfied Section		
						Print A	l Properti	es	
Section	CHK	SEL	LCB	COM	SHR	Н	В	AREA	
HEM220	OK		2	0.820	0.150	0.2400	0.2260	0.0149	
HEA320	OK		2	0.826	0.165	0.3100	0.3000	0.0124	
IPN400	OK		2	0.955	0.109	0.4000	0.1550	0.0118	
HEB260	OK		2	0.916	0.181	0.2600	0.2600	0.0118	
HEA300	OK		2	0.961	0.178	0.2900	0.3000	0.0113	
IPE450	OK		2	0.969	0.132	0.4500	0.1900	0.0099	